JANUARY 2011 FLOOD EVENT
# TABLE OF CONTENTS

1 INTRODUCTION........................................................................................................................................1

2 WIVENHOE DAM FLOOD MITIGATION AND FLOOD OPERATIONS ..................................................2
   2.1 Flood Mitigation ................................................................................................................................2
   2.2 Flood Operations ............................................................................................................................3

3 MANUAL OF FLOOD MITIGATION FOR WIVENHOE AND SOMERSET DAMS ......4

4 JANUARY 2011 FLOOD EVENT........................................................................................................6
   4.1 Background ......................................................................................................................................6
   4.2 Event Decision Making ...................................................................................................................7
   4.3 Flood Mitigation Benefits of Wivenhoe Dam ..................................................................................10

5 EVENT REVIEW ....................................................................................................................................12
1 INTRODUCTION

Wivenhoe Dam was constructed by the Queensland Government between 1977 and 1984. The dam is a 56 m AHD high and 2.3 kilometre long earth and rock embankment separated into two parts by a concrete gravity spillway. The spillway is controlled by 5 radial gates, each 12.0 metres wide by 16.0 m AHD high. Two saddle dam embankments are located on the left side of the reservoir.

The dam spillway capacity was upgraded in 2005. This was done primarily through the construction of a 164 metre wide secondary spillway through the right abutment of the existing dam. This spillway contains three erodible earth fill fuse plug embankments that are initiated at different dam levels in excess of EL 75.6.

The dam has two main functions by providing:

- A 1,165,000 ML storage at full supply level (FSL EL 67.0) providing an urban water supply for Brisbane and surrounding areas;

- Flood mitigation in the Brisbane River by providing a dedicated flood storage volume of 1,450,000 ML up to EL77 (this flood level was increased as part of the 2005 upgrade to allow a water level of EL80m and a temporary flood storage volume of 1,966,000 ML with all fuse plugs initiated and the dam at the point of failure).

The dam has an EXTREME hazard classification under ANCOLD guidelines because of the significant development downstream in the Brisbane and Ipswich metropolitan areas, with the population at risk in the event of a dam failure numbering in the hundreds of thousands.

In accordance with the Queensland Regulatory program for dam spillway upgrades, a further upgrade of Wivenhoe Dam for dam safety reasons only is scheduled to occur prior to 2035 to enable the dam to safely pass the Probable Maximum Flood. This work will involve the reconstruction of Saddle Dam 2 as a fuse plug spillway.

Wivenhoe Dam is in excellent condition. Comprehensive Dam Safety reviews undertaken in accordance with ANCOLD guidelines have been undertaken in 1997 (Gutteridge, Haskins & Davey Pty Ltd), 2003 (Wivenhoe Alliance), 2006 (NSW Department of Commerce), 2009 (GHD) and September 2010 (Seqwater). The reports concluded that the design of the dam is in accordance with modern day standards and that there are no significant outstanding design or construction issues that require investigation.
2 WIVENHOE DAM FLOOD MITIGATION AND FLOOD OPERATIONS

2.1 Flood Mitigation

The Brisbane River catchment covers an area of approximately 14,000 square kilometres of which about half is below Wivenhoe Dam. Maximum overall flood mitigation effect is achieved by operating Wivenhoe Dam in conjunction with Somerset Dam. Although Somerset and Wivenhoe Dam reduce flooding in Brisbane City, major flooding can still occur. The Lockyer-Laidley Valley drains into the Brisbane River through Lockyer Creek that enters the Brisbane River just downstream of Wivenhoe Dam near Lowood. Another major tributary, the Bremer River, flows into the Brisbane River at Moggill. Wivenhoe Dam has no control over inflows into the Brisbane River from both these major tributaries.

Wivenhoe Dam mitigates downstream flooding by storing incoming flood water during a rainfall event and releasing these waters at a reduced flow rate downstream to minimise flood impacts. The timing of the releases is also manipulated so that the aim is for outflows from the dams to impact on downstream areas only after the peak inflows from the downstream major tributaries have passed. However, this aim cannot always be achieved in practice. This is because some large floods, such as the one currently being experienced, have the potential to overflow the dam’s flood storage compartment. Should this occur, the dam would fail and the resulting damage and loss of life would be at least 100 to 1000 times greater than that currently being experienced.

Therefore the basis of all flood operation decision making is to ensure the dam never fails. This is the reason that the dam’s flood storage compartment would never be intentionally fully filled as additional inflows after this point would result in a dam failure. Similarly, there will be uncertainty on future rainfall that could occur which could not be releases if there was insufficient flood storage which could not be stored or released.

Another factor that impacts on flood release decision making in large events are the levels at which the erodible fuse plugs are triggered. Loss of one or more fuse plugs severely limits the ability of the dam to mitigate the effects of future flood events that may occur prior to the fuse plug or plugs being reinstated. Reinstatement of a fuse plug following an event would take a minimum of 4 to 6 months and would require an extended period of relatively dry weather.
2.2 Flood Operations

A real time flood monitoring and forecasting system has been established in the Wivenhoe and Somerset Dam catchments. This system employs radio telemetry to collect, transmit and receive rainfall and stream flow information. The system consists of around 230 field stations that automatically record rainfall and/or river heights at selected locations in the dam catchments. Most of these field stations are owned by Seqwater with the remainder belonging to other agencies.

The rainfall and river height data is transmitted to Seqwater’s Flood Operations Centre in real time. Once received in the Flood Operations Centre, the data is processed using a Real Time Flood Model (RTFM) to estimate likely dam inflows and evaluate a range of possible inflow scenarios based on forecast and recorded rainfall in the dam catchments. The RTFM is a suite of hydrologic computer programs that utilise the real time data to assist in the operation of the dams during flood events.

Seqwater engineers use the RTFM for flood monitoring and forecasting during flood events to operate the dams in accordance with a Manual of Flood Mitigation (the origin of and objectives and procedures contained in the Manual of Flood Mitigation are explained in the following section of this document). Releases of water from the dams are optimised to minimise the impacts of flooding in accordance with the objectives and procedures contained in a Manual of Flood Mitigation.

The RTFM and data collection network performed well during the January 2011 event, with no failures experienced that compromised the ability of Seqwater to operate the dam.
3 MANUAL OF FLOOD MITIGATION FOR WIVENHOE AND SOMERSET DAMS

The Manual of Flood Mitigation for Wivenhoe and Somerset Dams, in its current form, was developed in 1992 during an extensive hydrological study of the Brisbane and Pine Rivers catchments by DPI, Water Resources. The final reports were subject to extensive internal review by the Water Resources Group before being reviewed by an independent review panel comprising Professor Colin Apelt, Head of Department, Department of Civil Engineering, University of Queensland and Mr Eric Lesleighter, Principal Hydraulic Engineer and Chief Engineer Water Resources, Snowy Mountains Engineering Corporation.

Subsequently, the Manual was extensively reviewed during the Brisbane Valley Flood Damages Minimisation Study in 2006, with the latest comprehensive review of the Manual undertaken in 2009. Both of these reviews have included expert review panels comprising key stakeholders, with the most recent review involving representatives from DERM, BOM, BCC and SunWater.

The Manual of Flood Mitigation is prepared by Seqwater as the owner of the dam and approved and gazetted by the Chief Executive of DERM in accordance with the Water Supply Act 2008. The manual defines flood objectives procedures; roles and responsibilities; and staffing and operational requirements for flood events impacting on Wivenhoe and Somerset dams.

The primary objectives of the procedures contained in the flood manual are, in order of importance:

- Ensure the structural safety of the dams;
- Provide optimum protection of urbanised areas from inundation;
- Minimise disruption to rural life in the valleys of the Brisbane and Stanley Rivers primarily, this involves minimising inundation of the seven bridges below the dam upstream of Moggill);
- Retain the storage at Full Supply Level at the conclusion of the Flood Event.
- Minimise impacts to riparian flora and fauna during the drain down phase of the Flood Event.

During an event, the operation of the dam transitions between the following four operating strategies depending of the circumstances at the time. These procedures associated with these strategies are explained in detail in the Manual.
- **Strategy W1** – Primary consideration is given to Minimising Disruption to Downstream Rural Life. Under this strategy, the predicted water level is below 68.50 m AHD and the maximum release is 1,900m3/s.

- **Strategy W2** – Transition Phase moving from Minimising Disruption to Protecting Downstream Urban Areas. Under this strategy, the water level is predicted to be between 68.5 and 74.0 m AHD and the maximum release is less than 3,500m3/s.

- **Strategy W3** – Primary consideration is to Protect of Urban Areas from Inundation. Under this strategy, the water level is predicted to be between 68.5 and 74.0 m AHD but the maximum release is less than 4,000m3/s.

- **Strategy W4** – Primary consideration is to protecting the structural safety of the Dam. Under this strategy, the water level is predicted to exceed 74.0 m AHD and there is no limit to the maximum release. Consideration is given to managing flood releases to avoid fuse plug initiation if at all possible as this would compromise flood mitigation capacity in the short to medium term.

In addition to these strategies, historical records show that there is a significant probability of two or more flood producing storms occurring in the Brisbane River system within a short time of each other. Accordingly for each flood event, the aim is always to empty stored floodwaters within seven days after the flood peak has passed through the dams.
4 JANUARY 2011 FLOOD EVENT

4.1 Background

In the 25 days leading up to the current event, three flood events impacting on Wivenhoe Dam were experienced, with gate releases being made on all but five of those days. The total outflow from these events was around 700,000ML. The details of these events are as follows:

<table>
<thead>
<tr>
<th>EVENT</th>
<th>EVENT START DATE</th>
<th>EVENT END DATE</th>
<th>VOLUME RELEASED (ML)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>13/12/2010</td>
<td>16/12/2010</td>
<td>70,000</td>
</tr>
<tr>
<td>2</td>
<td>17/12/2010</td>
<td>24/12/2010</td>
<td>150,000</td>
</tr>
<tr>
<td>3</td>
<td>26/12/2010</td>
<td>02/01/2010</td>
<td>470,000</td>
</tr>
</tbody>
</table>

During these events, requests were received from Councils and residents impacted by bridge closures downstream of the dam to curtail releases as soon and as quickly as possible. Additionally the 2 January end date of the flood event prior to the current event meant that significant drain down of the dam prior to the onset of the current event that commenced on 6 January 2011, was not possible without major bridge inundation downstream of the dam and without exceeding minor flood levels in the lower Brisbane River.

Additionally, a flood event was also experienced in October 2010 that resulted in a release of 750,000ML from the dam. Accordingly drain down below the dam full supply level prior to the start of the first December event would not have been possible without significant bridge inundation and without exceeding minor flood levels (as defined by BOM and BCC) in the lower Brisbane River.

Regardless, significant drain down prior to the current event would have had little impact on the peak level in Wivenhoe Dam as shown in the table below. The reason for this is that this total event inflow volume of 2,600,000 ML is well in excess of the useable flood storage combined with the available water supply storages shown in the table.

The specific impact on the Lower Brisbane River of these reduced dam levels requires the use of a complex hydraulic model. The results of this modelling would still contain a degree of uncertainty as illustrated by the difficulties in estimating the final flood peak in Brisbane during the event. This
is because the rapid closure of the gates after peak inflow was achieved resulted in significant water level reductions downstream and this is difficult to model accurately.

<table>
<thead>
<tr>
<th>JANUARY 2011 FLOOD</th>
</tr>
</thead>
<tbody>
<tr>
<td>Starting Level %</td>
</tr>
<tr>
<td>100</td>
</tr>
<tr>
<td>95</td>
</tr>
<tr>
<td>90</td>
</tr>
<tr>
<td>75</td>
</tr>
<tr>
<td>50</td>
</tr>
</tbody>
</table>

It should be noted that the possible reductions shown above are based on a dual peaked flood hydrograph with a volume of about 2,600,000 ML which occurred during this event. A hydrograph with the same volume but a different distribution could result in a significantly lower reduction in peak water levels. Flood operations at the dam are also highly dependent upon the flood inflow volume and a slight variation in the flood volume could significantly reduce the benefits associated with draining down the dam prior to a flood event.

### 4.2 Event Decision Making

The following table contains a summary of the key decisions points associated with the current event. As at 16 January 2011, the event remains in progress.

<table>
<thead>
<tr>
<th>DATE AND TIME</th>
<th>FLOOD EVENT MILESTONE</th>
</tr>
</thead>
<tbody>
<tr>
<td>07:00 06/01/2011 (Thursday)</td>
<td>Rainfall is experienced in the dam catchments that will result in flood releases, however Wivenhoe releases are delayed for 24 hours to allow Lockyer Creek flood flows to pass downstream and prevent the isolation of the community dependent of Burtons Bridge. The forecast is for 150mm over the next 24 hours.</td>
</tr>
<tr>
<td>15:00 07/01/2011 (Friday)</td>
<td>Wivenhoe releases commence, with operational strategy W1 in use. Rainfall for the next four days is estimated to be between 140mm and 300mm, with a forecast for rain easing on Tuesday 11 January 2011. All bridges downstream of the dam with the exception of Fernvale Bridge and Mt Crosby Weir Bridge are expected to be inundated for a number of days.</td>
</tr>
<tr>
<td>Time</td>
<td>Date</td>
</tr>
<tr>
<td>--------</td>
<td>------------</td>
</tr>
<tr>
<td>06:00</td>
<td>09/01/2011</td>
</tr>
<tr>
<td>15:30</td>
<td>09/01/2011</td>
</tr>
<tr>
<td>06:30</td>
<td>10/01/2011</td>
</tr>
<tr>
<td>06:30</td>
<td>10/01/2011</td>
</tr>
<tr>
<td>08:00</td>
<td>11/01/2011</td>
</tr>
<tr>
<td>11:00</td>
<td>11/01/2011</td>
</tr>
<tr>
<td>21:00</td>
<td>11/01/2011</td>
</tr>
<tr>
<td>Time</td>
<td>Date</td>
</tr>
<tr>
<td>---------</td>
<td>------------</td>
</tr>
<tr>
<td>22:00</td>
<td>11/01/2011</td>
</tr>
<tr>
<td>08:00</td>
<td>12/01/2011</td>
</tr>
<tr>
<td>21:00</td>
<td>13/01/2011</td>
</tr>
<tr>
<td>09:00</td>
<td>17/01/2011</td>
</tr>
</tbody>
</table>
4.3 Flood Mitigation Benefits of Wivenhoe Dam

The following graphs demonstrate the significant benefits of Wivenhoe Dam in mitigating the current flood event, with reductions in flood peak of up to 2.5 metres in the City area and up to 5.5 metres in the Moggill area further upstream.

This equates to significant reduction in the potential for loss of life as well as saving in damages in the order of up to $1.6 billion based on current damage curves. Up to 13,000 more properties would have been impacted by the event without the Dam.

The time at which flood levels remained elevated above major levels has also been reduced by up to 3 days by the dam. This has significant benefits to impact on the population of the city, property damage and the recovery operation.
The strategy adopted to quickly close off releases once the peak in the dam had been reached and rain stopped falling certainly reduced the predicted flood peak by at least one metre in the lower Brisbane River area. This was carried out because the releases had stopped the dam from rising and careful monitoring allowed rapid reduction of releases while ensuring fuse plug initiation did not occur.

This notion is supported by BOM.
5 EVENT REVIEW

Under the Manual of Operational Procedures for Flood Mitigation at Wivenhoe Dam and Somerset Dam that are approved and gazetted by the Department of Environment and Resource Management, there is a regulatory requirement that a report must be prepared as per the below wording:

“Seqwater must prepare a report after each Flood Event. The report must contain details of the procedures used, the reasons therefore and other pertinent information. Seqwater must forward the report to the Chief Executive within six weeks of the completion of the Flood Event.”

Such a report was prepared for the flood events of February and March 2010 and copies are available. A copy of the Table of Contents of that report is included as Appendix 1. For this event, the report would be a comprehensive summary of all procedures, actions, outcomes and processes during the event.

It is recommended that the process and content for reports required for this event be:

- In the short term, utilise this report attached to this briefing note as the basis for communications and discussion.
- Prepare any Interim Reports as agreed to provide information and input as required.
- Seqwater prepare a Comprehensive Report as per the existing regulatory requirements of the Act and the gazetted manual and any requirements of the Dam Safety Regulator. This would be done within 6 weeks of the closure of the current event as per the manual. This timeframe is subject to any new mobilisation of the Flood Operations Centre. The Table of Contents would include:
  - Introduction
  - Flood Event Summary
  - Mobilisation and Staffing
  - Event Rainfall
  - Inflow and Release Details
  - Data Collection System Performance
  - Data Analysis Performance
  - Communication
  - Flood Management Strategies and Manual Compliance
• Improvements in data collection systems, practices and processes.
• Improvements by interacting agencies
• Review of factors impacting on the protection of urban areas
• Recommendations & Conclusions

• The report would then be reviewed by the Dam Safety Regulator in conjunction with any peer review they require. The review should cover:
  • Were the provisions of the manual complied with?
  • What improvements to either facilities e.g. stream gauges, or work practices, are desirable to improve Sewater's ability to predict inflows into the dams.
  • Are improvements to either Seqwater's facilities or work practices desirable to improve Seqwater's ability to manage events? For example, investigations to raise the dam to improve its flood storage capacity, If so, what are they and their implications.
  • Are changes to the facilities or work practices of other organisations desirable to improve Seqwater's abilities to manage these events?
  • Whether it is worth investigating increasing the flood capacity of Wivenhoe
  • Whether the Brisbane River crossings which act, under some situations as a constraint on the releases from Wivenhoe, should be replaced by bridges. For example if the smallest could pass , for example, 2,500 cumecs, then this could enable higher releases under some circumstances.
  • Whether the policy of draining the flood compartment within 7 days should be modified.
  • Given the manual's order of priorities i.e. protection of the dam etc, are any changes in the flood release strategies for either dam desirable? If so, what are they, and their implications

• Based on this review, a review of the Manual of Operational Procedures for Flood Mitigation at Wivenhoe Dam and Somerset Dam would occur utilising an expert panel of review including representatives of DERM, Seqwater, BoM, affected Local Governments and other stakeholders as necessary.
Appendix A
TABLE OF CONTENTS

1.0 INTRODUCTION ........................................................................................................ 1

2.0 FLOOD EVENT SUMMARY ........................................................................................ 2
  2.1 FEBRUARY 2010 ........................................................................................................ 2
  2.2 MARCH 2010 ............................................................................................................. 3

3.0 MOBILISATION AND STAFFING ..............................................................................5
  3.1 16TH TO 18TH FEBRUARY 2010 .............................................................................. 5
  3.2 1ST TO 4TH MARCH .................................................................................................. 5
  3.3 5TH TO 6TH MARCH ................................................................................................ 5
  3.4 6TH TO 7TH MARCH ................................................................................................ 6
  3.5 10TH TO 11TH MARCH ............................................................................................. 6

4.0 EVENT RAINFALL ..................................................................................................... 7
  4.1 FEBRUARY 2010 ....................................................................................................... 7
  4.2 MARCH 2010 ........................................................................................................... 9

5.0 INFLOW AND RELEASE DETAILS ........................................................................ 12
  5.1 16TH TO 18TH FEBRUARY 2010 .......................................................................... 12
  5.2 1ST MARCH 2010 .................................................................................................... 15
  5.3 6 / 7TH MARCH 2010 .............................................................................................. 18
  5.4 11TH MARCH 2010 ................................................................................................ 21

6.0 DATA COLLECTION SYSTEM PERFORMANCE ..................................................... 25
  6.1 NETWORK AVAILABILITY ......................................................................................... 25
  6.2 RAINFALL .................................................................................................................. 25
  6.3 STREAM HEIGHT ....................................................................................................... 26
    6.3.1 Main Stream ........................................................................................................ 26
    6.3.2 Back-up Stream ................................................................................................... 27
    6.3.3 Headwater Sensors ............................................................................................ 27
    6.3.4 Sensor Calibration ............................................................................................. 28
    6.3.5 Stream Height Rating Curves ............................................................................. 28

7.0 DATA ANALYSIS PERFORMANCE ....................................................................... 29
7.1 RUNOFF-ROUTING MODELS ................................................................. 29
7.2 BASEFLOW ASSESSMENT ................................................................. 29

8.0 COMMUNICATIONS ............................................................................ 31
8.1 EVENT LOG ....................................................................................... 31
8.2 DECLARATION OF EVENT MOBILISATION ...................................... 31
8.3 SHIFT HANOVER .............................................................................. 32

9.0 FLOOD MANAGEMENT STRATEGIES .............................................. 33
9.1 16TH TO 18TH FEBRUARY 2010 ....................................................... 33
  9.1.1 North Pine Dam ............................................................................. 33
9.2 26TH FEBRUARY TO 4TH MARCH .................................................... 35
  9.2.1 Somerset and Wivenhoe Dam ......................................................... 35
  9.2.2 North Pine Dam ............................................................................. 38
9.3 4TH MARCH TO 7TH MARCH ............................................................ 41
  9.3.1 North Pine Dam ............................................................................. 41
9.4 10TH MARCH TO 11TH MARCH ....................................................... 44
  9.4.1 North Pine Dam ............................................................................. 44

10.0 RECOMMENDATIONS & CONCLUSIONS ........................................... 46
10.1 DATA COLLECTION .......................................................................... 46
10.2 DATA ANALYSIS ............................................................................. 46
10.3 DIRECTIVES AND STATUS REPORTS ............................................. 46
10.4 OPERATIONAL STRATEGIES ............................................................ 47

LIST OF APPENDICES

APPENDIX A – FOC Attendance Logs & Event Logs
APPENDIX B – Estimated Inflows and Release
APPENDIX C – Reservoir Headwater Levels

LIST OF TABLES

Table 2.1: Storage Status Prior to February 2010 Event ............................... 2
Table 2.2: Dam Operations in March 2010 .................................................. 3
Table 4.1: February 2010 Sub-Catchment Rainfalls (mm) ............................. 7
Table 4.2: March 2010 Sub-Catchment Rainfalls (mm) ................................. 9
Table 5.1: Peak Flood Level and Flows – 16th to 18th February 2010 .......... 12
Table 5.2: Somerset Dam – 16th February to 20th February 2010 ................. 14
Table 5.3: Wivenhoe Dam – 16th February to 20th February 2010 .................................................14
Table 5.4: North Pine Dam – 16th February to 18th February 2010 ........................................14
Table 5.5: Peak Flood Level and Flows – 1st March 2010 ............................................................15
Table 5.6: Somerset Dam – 24th February to 18th March 2010 .............................................17
Table 5.7: Wivenhoe Dam – 24th February to 18th March 2010 .............................................17
Table 5.8: North Pine Dam – 26th February to 4th March 2010 .............................................18
Table 5.9: Peak Flood Level and Flows – 6/7th March 2010 ......................................................19
Table 5.10: North Pine Dam – 4th March to 6th March 2010 .............................................21
Table 5.11: Peak Flood Level and Flows – 11th March 2010 ....................................................22
Table 5.12: North Pine Dam – 10th March to 11th March 2010 ............................................24
Table 9.1: North Pine Dam Gate Settings – 17th to 18th February 2010 .................................35
Table 9.2: Somerset Dam gate Settings – 24th February 18th March .................................37
Table 9.3: North Pine Dam Gate Settings – 1st to 4th March 2010 ........................................39
Table 9.4: North Pine Dam Gate Settings – 5th to 7th March 2010 .........................................42
Table 9.5: North Pine Dam Gate Settings – 10th to 11th March 2010 .................................45