IN THE MATTER OF THE QUEENSLAND FLOODS COMMISSION OF INQUIRY

A COMMISSION OF INQUIRY UNDER THE COMMISSIONS OF INQUIRY ACT 1950

AND PURSUANT TO COMMISSIONS OF INQUIRY ORDER (No. 1) 2011

SECOND STATEMENT OF TERRENCE ALWYN MALONE

On the 11th day of April 2011, I, Terrence Alwyn Malone of 240 Margaret St, state on oath:

1. I am employed by Queensland Bulk Water Supply Authority (*Seqwater*) in the position of Principal Hydrologist, Water Delivery.

Impact of increased releases on Sunday, 9 January 2011

- 2. I am aware of a suggestion that if Wivenhoe releases were increased to 3,000m³/s at midnight on Sunday, 9 January 2011 the peak of the flood in the lower Brisbane River (Moggill) would have been significantly lower.
- 3. I have since undertaken a modelling analysis to investigate this suggestion and I make the following comments:
 - (a) A gauging or flow measurement was undertaken by a joint Seqwater and Department of Environment and Resource Management hydrographic team on the evening of 12 January 2011 from Jindalee Bridge, just downstream of Moggill. In practice, the peak flow at Jindalee is the same as that at Moggill. The flow measured at this location around the peak of the event was about 9,800m³/s.
 - (b) A hydrologic model of the catchment adopting the Wivenhoe actual release hydrograph gives 9,300m³/s at Jindalee, which is reasonably consistent with the magnitude and timing of the measured peak.
 - (c) For modelling purposes, the outflow from Wivenhoe Dam was modified as indicated in the graph below. Releases were increased from 1,450m³/s at 0900 Sunday, 9 January 2011 to 3,000m³/s at 0000 Monday, 10 January 2011 and continued to be increased until 1500 Tuesday, 11 January 2011. I note that these releases would be contrary to the manual but they have been adopted for modelling purposes.

Filed on behalf of: Queensland Bulk Water Supply Authority trading as Seqwater

Allens Arthur Robinson Lawyers Riverside Centre 123 Eagle Street Brisbane QLD 4000

DX 210 Brisbane Tel (07) 3334 3000 Ref MGI:120128021

- (d) Under this scenario, at 1500 Tuesday, 11 January 2011 the water level in the dam would have reached EL 74.0m AHD and strategy W4 would have been invoked. Under the current manual, releases would then have been increased up to 5,000m³/s, at which time the modelling shows that water levels would have stabilised.
- (e) The release strategy would then have reverted to that adopted during the January 2011 flood event.
- (f) The modelled release of water from Wivenhoe dam is shown in Figure 1. The reduction in peak discharge from Wivenhoe dam is apparent by comparing the peak of the solid blue line (which is the actual release during the 2011 flood event) with the peak of the dotted blue line (which is the modelled scenario I have outlined above). The volume under the solid blue and dotted blue lines is the same (that is, 2,650,000ML).

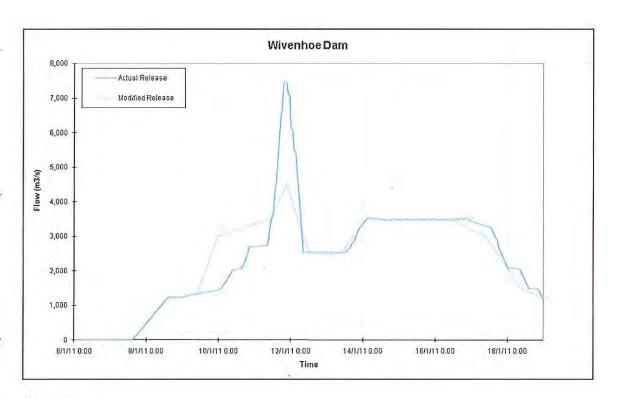


Figure 1

(g) The modelled impact of this modified release strategy upon the flow at Moggill is shown in Figure 2. I note that the reduction in peak flow is only small, but the onset of damaging flows of 4,000m³/s as set out in the manual is earlier and lasts for about 12 hours longer.

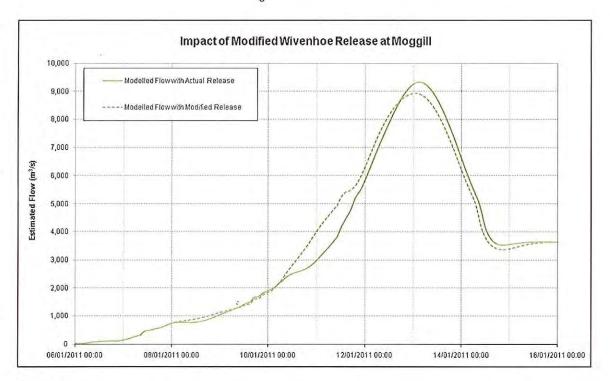


Figure 2

(h) Figure 3 below shows that the modelled impact of releasing earlier from Wivenhoe dam as has been suggested would have had minimal impact on the peak height at the Brisbane Port Office gauge.

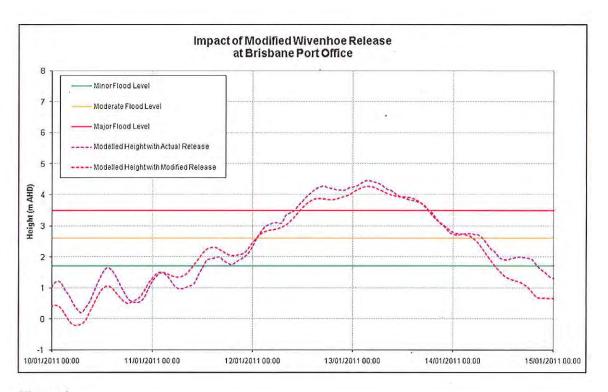


Figure 3

- (i) It should be noted that these results are based upon a hydrologic model, when a hydrodynamic model would be more appropriate and expected to give more accurate results. I do not have a working hydrodynamic model but Seqwater have commissioned SKM to get the Hydrodynamic model used by the Wivenhoe Alliance Spillway Augmentation Study working. However, the results are indicative of the relative impact of the suggested change in the release strategy.
- (j) Even though the Wivenhoe Dam peak release may have been reduced from 7,500m³/s to under 5,000m³/s under the early release suggestion, the impact in the lower Brisbane River (Moggill) is minimal. This is largely due to the interaction between the main river channel and its adjacent floodplain at several locations along the river between Wivenhoe Dam and Brisbane. This behaviour is driven as much by flood volumes as flood peaks. In the case of the early release strategy, flood volumes do not change. The flood peaks of the Lockyer River, Bremer River and local area runoff remains unchanged under the early release suggestion, hence the minimal impact in the lower reaches.

Impact if Wivenhoe dam started the January 2011 Flood Event at 75% of Full Supply Level

- 4. I am also aware of a suggestion that if the level in Wivenhoe Dam was at 75% of Full Supply Level (EL 64m AHD) at the commencement of the January 2011 Flood Event, then the peak of the flood in the lower Brisbane River (Moggill) would have been significantly lower.
- 5. I deal with this suggestion below:
 - (a) At 75% capacity, equivalent to about EL 64m AHD, Wivenhoe Dam has about 290,000ML to the Full Supply Level of EL 67.0m AHD and an additional 28,000ML before reaching the gate trigger level of EL 67.25m AHD, making a total deficit of 318,000ML. It should be noted that this volume deficit is only 12% of the total volume of the inflow to Wivenhoe Dam during the January 2011 flood.
 - (b) Modelling of inflows to the dam shows that this deficit would have been filled and gate trigger level reached on early Sunday morning without any releases up to this time.
 - (c) From this point releases would have been made in accordance with the Manual.

 Releases would have been increased up to 1,500 m3/s late Sunday night and thereafter would have followed a similar pattern to the actual releases up to 1400 Tuesday 11th January 2011.

- (d) At this point in time, the water level in the dam would have reached EL 74.0m AHD and strategy W4 would have been invoked. Releases would then have been increased until water levels stabilised, which the modelling shows would have occurred at a peak flow of 6,500 m3/s. Peak height in Wivenhoe Dam may have been lower by about 0.7m.
- (e) The release strategy would then have reverted to that adopted during the event.
- (f) The outflow from Wivenhoe dam for this scenario would have been as indicated in Figure 4 below. The solid blue line is actual releases and the dotted blue line is the modelled releases under the scenario I have explained. The volume under the solid blue and dotted blue lines is the same (that is, 2,650,000ML).

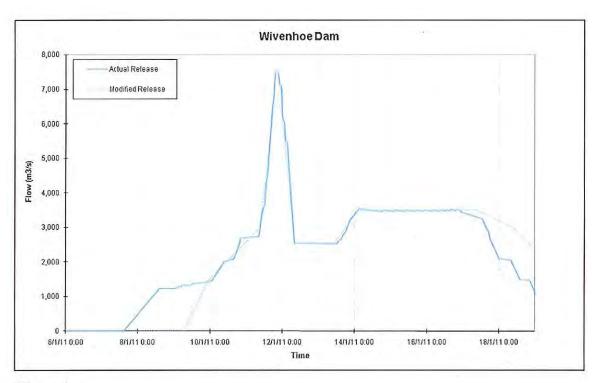


Figure 4

(g) The modelled impact of this modified release strategy is shown in Figure 5. The modelled reduction in peak flow at Moggill is very small. The reduction in starting level only impacts the low flows between 8th and 11th January. After this point, the hydrograph at Moggill is virtually the same.

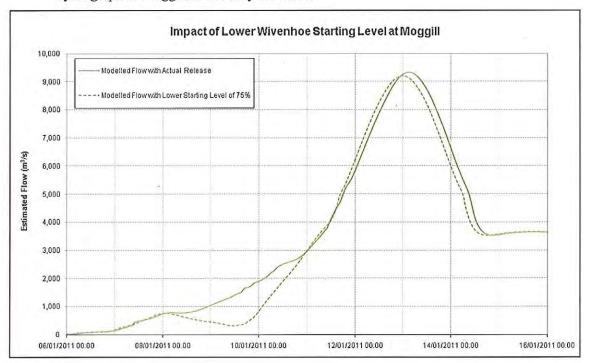


Figure 5

(h) At the Brisbane Port Office Gauge, Figure 6 below shows that the impact of a lower starting level at Wivenhoe dam would have had minimal impact on the peak height at the Brisbane Port Office Gauge.

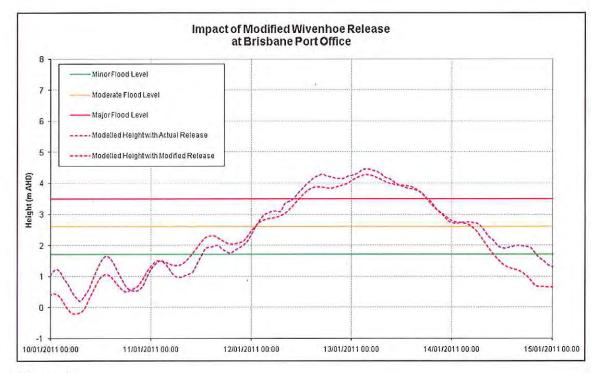


Figure 6

- (i) Again, it should be noted that these results are based upon a hydrologic model when a hydrodynamic model would be more appropriate and expected to give more accurate results. However, the results are indicative of the relative impact of the suggested change in the release strategy.
- (j) Under the scenario of a lowering starting level of 75% at Wivenhoe, the peak release may have been reduced from 7,500m3/s to 6,500m3/s but the impact in the lower Brisbane River is minimal because of the size of the January 2011 Flood Event and reasons outlined in 3(j). Further, the flood peaks of the Lockyer, Bremer and local area runoff remains unchanged.

Wivenhoe Headwater Gauge readings

6. I was recently requested by Peter Allen of DERM to provide information relating to the differences in gauge board and ALERT readings of the lake level in Wivenhoe Dam during the January 2011 Flood Event. Attached as Annexure TM-8 is a paper I prepared, which is true.

My first statement

- 7. I refer to:
 - (a) paragraphs 66, 68(a) and 69 of my first statement dated 25 March 2011; and
 - (b) my interview with Lisa Hendy of the Commission of Inquiry on 30 March 2011 and, in particular, my statement contained in lines 20 to 21 on page 10 of the transcript of my interview.
- 8. Since making my first statement, I have been shown an email I sent on 25 October 2010 to certain individuals from Seqwater, the Bureau of Meteorology (the *BOM*) and the Brisbane City Council (the *Council*). A copy of that email is **Annexure TM-9**.
- 9. At the time of making my first statement and attending my interview with Commission staff, I had forgotten the email shown in **Annexure TM-9**.
- 10. The context of that email is as follows:
 - (a) There had been a Flood Event earlier in October 2010. Low level releases had been made from Wivenhoe dam during the event and there had been some confusion in public statements by agencies about whether the releases would cause low level flooding in Brisbane.
 - (b) Following the October Flood Event, Seqwater, the BOM and the Council commenced discussions about what communications should occur between the parties in respect of future releases from Wivenhoe Dam.

- (c) I received an email on 18 October 2010 regarding a meeting to be held on 22 October 2010 between Seqwater, BOM and the Council. Annexure TM-10 is a copy of that email.
- I presented a powerpoint at the meeting on 22 October 2010. Annexure TM-11 is a copy of that powerpoint. The sixth page of the powerpoint is a copy of Appendix B of the manual of operational procedures for Wivenhoe and Somerset Dams.
- (e) I cannot now recall the discussions which occurred at the meeting on 22 October 2010. I made notes during the meeting. A copy is shown in **Annexure TM-12**.
- (f) Although I cannot now recall the discussions which occurred at the meeting, based on my notes, I accept it is possible that Council officers said to me that their view was that urban damage would occur in Brisbane at a rate of 3,500 cumecs.
- (g) On 25 October 2010 I proposed a set of triggers, as shown in the email Annexure TM-9, which would used to initiate communications between Sequater and other parties.
- (h) I also attended a meeting on 29 October 2010. I cannot now remember the discussions in that meeting.
- 11. As a result of my above comments, it may be that the statement at paragraph 66 of my first statement is incorrect.
- 12. I note in answer to a question from the Commission (at lines 20 to 21 on page 10 of the transcript of my interview with the Commission) I said I was surprised at the information provided by Council earlier in the night to the other engineers. I do recall being surprised at the time. That may have been because I had forgotten my email and the earlier discussion and was focussed on the current event.
- 13. At no time during these discussions did anything the Council say trigger in my mind any thought it was necessary to amend Strategy W3 in the Manual.
- 14. My note shown in **Annexure TM-12** says "review Q (which means flow) in Table in Appendix of manual". I made no note to review Strategy W3 in the Manual.

SWORN by TERRENCE ALWYN MALONE on 11 April 2011 at Brisbane in the presence of:

Deponent	Solicitor

IN THE MATTER OF THE QUEENSLAND FLOODS COMMISSION OF INQUIRY

A COMMISSION OF INQUIRY UNDER THE COMMISSIONS OF INQUIRY ACT 1950

AND PURSUANT TO COMMISSIONS OF INQUIRY ORDER (No. 1) 2011

SECOND STATEMENT OF TERRENCE ALWYN MALONE

INDEX OF ANNEXURES

Annexure No.	Document	Date
TM-8	Paper regarding lake level readings	
TM-9	Email from me to the BCC and the BOM	25/10/2010
TM-10	Email from BCC	18/10/2010
TM-11	Powerpoint presentation	22/10/2010
TM-12	Note	22/10/2010

Wivenhoe Headwater Gauge

The headwater level of Wivenhoe Dam is measured is three ways during a flood event:

- Gauge boards which located at the back of left hand training wall in an area of still water.
- 2. A float well and shaft encoder (ALERT Id 6637) located about 35 metres upstream of the gates on the left hand side of the approach to the gates.
- 3. A wet pressure transmitter (ALERT Id 6638) located in the area behind the trash racks on the left hand side of the approach to the gates.

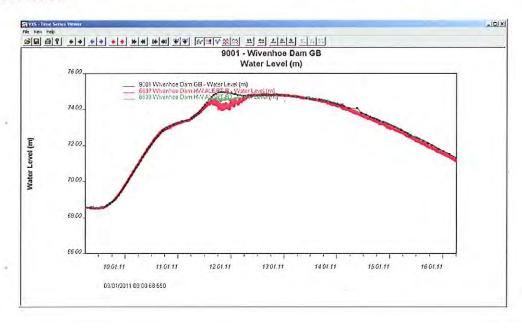
Outside of floods, gauge boards, a short drive from the dam office, are used. These are not used during floods as this would require the operators to leave the site. Normally, gauge board readings were sent in by email at the commencement of each day. During flood events, readings are emailed and faxed at regular hourly intervals. At the peak of the Jan 2001 flood, half hourly readings were phoned directly into the FOC.

ALERT Ids 6637 and 6638 are event based radio telemetry stations which report every event (i.e. incremental change in water level) via radio directly to the FOC, BoM and some Councils.

ALERT gauge 6638 was published on the BoM web site which is updated with the latest information every 15 minutes.

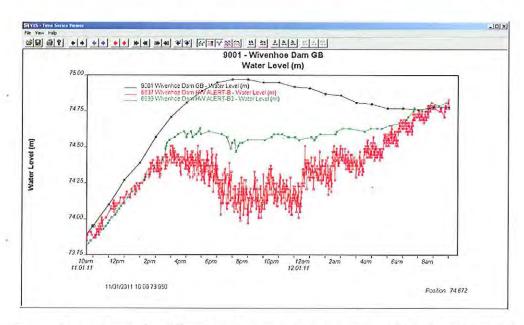
Traditionally, float well and shaft encoders are preferred over pressure transmitters as they have proven to be more reliable than wet pressure transmitters, especially in situations where a wide range levels is monitored.

A comparison of the three water level sensors for the period 10th to 16th January 2011 is shown below.



The figure above clearly shows that there is a significant difference between the gauge board readings and the ALERT gauges during the period around the peak.

The Seqwater Senior Hydrographer advised the FOC of the discrepancy between the gauge board readings and the automatic gauges early in the afternoon of 11th January 2011. Up to this time, there was little difference between the readings. An email was issued to BoM and BCC advising of the discrepancy at 2.26pm 11th January 2011.



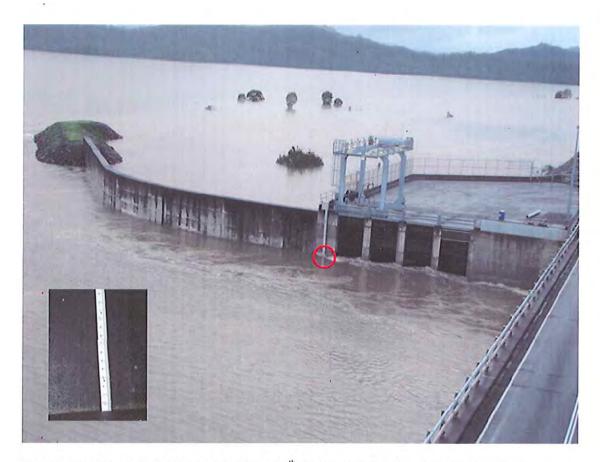
The figure above shows the differences between the automatic readings and the gauge board readings for the 11th January 2011.

ALERT gauge 6638, shown in red, is very close to the gauge board readings. However, as the outflows increased during the afternoon of the 11th January, the gauge commenced to oscillate over a range of 200mm in very short time. It under reads the gauge boards by a much as 1 metre around the peak of the outflow.

ALERT gauge 6637, shown in green, is more stable but still under reads the gauge board readings by as much as 0.5 metres around the peak of the outflow.



This picture above was taken at 5.42pm 10th January 2011. At this time, the automatic headwater gauge (ALERT Id 6637) was reading 72.91m and the staff gauge reading was72.92m AHD. Note the laminar nature of the flow around the stilling well and the lack of turbulence. At the time, the outflow was the dam was 2,400m³/s.



The above picture was taken at 6.41pm 11th January 2011 about the time of the maximum height and release. At this time, the automatic headwater gauge (6637) was oscillating between 74.27 and 74.51m AHD and the staff gauge reading was 74.94m AHD. Release from the dam was about 6,770m3/s. Note the turbulent nature of the flow around the stilling well and the accompanying drawdown on the downstream side of the well. Also note the staff gauge reading (approximately 74.9m AHD) taken about this time.

Given the uncertainty associated with the ALERT gauges during the event, the operation of Wivenhoe Dam during the January 2011 flood was based upon the more reliable gauge board readings.

From:

James Charalambous

Sent:

Monday, 18 October 2010 4:01 PM

To:

Baddiley, Peter; J.Perkins

John Tibaldi; Terry Malone

Cc:

Carroll, Don; Caswell, Evan; Morris, Ken

Subject: Friday Morning 22/10 - Technical Capability Meeting

Ηi

As discussed this Friday 22/10 from 9am to 12:30pm would like to conduct a technical meeting between SEQWater/BOM/BCC:

Agenda as follows:

- 1. SEQ Water, BOM and BCC to each discuss for 30mins technical capabilities and how they operate/function in a flood event
- 2. Open discussion on communication between agencies, processing of information and data
- 3. Suggestions or improvements to current systems/processes

Morning tea and lunch will be provided.

Regards JamesC.		
Please direct all enquir	d through an insecure network. s to the message author.	
	Safe Stamp	

Your Anti-virus Service scanned this email. It is safe from known viruses. For more information regarding this service, please contact your service pl

From:

Terry Maione

Sent:

Monday, 25 October 2010 8:57 AM

To:

James Charalambous; Baddiley, Peter; Carroll, Don; Caswell, Evan; J.Perkins

stuar John Tibaldi; Morris, Ken;

Rob Drury

Cc:

Lavin, Chris; Scroope, Greg

Subject: RE: Friday Morning 29/10 - Technical Capability Meeting

Gents

Sequater would to submit this as a proposal for discussion at this Friday's meeting:

Relationships with external agencies during flood events when water is released from Wivenhoe Dam.

- Seqwater will contact Bureau of Meteorology (BOM), Somerset Regional Council (SRC), Ipswich City Council (ICC) and Brisbane City Council (BCC), Disaster Management Duty Officer, by phone when the Flood Operations Centre (FOC) is mobilised. It is understood that BCC Disaster Management Duty Officer will advise BCC Flood Information Centre (FIC)Duty Officer to establish an officer-to-officer technical relationship with the FOC.
- Seqwater will not contact ICC or BCC Councils unless releases are expected from Wivenhoe.
- Sequater may contact SRC if the expected flood impact is limited to areas upstream of Wivenhoe.
- 4. Segwater will advise Councils when it is expected that a combination of local runoff and releases from Wivenhoe will result in the following flows being reached or exceeded:

Trigger	Impact	Deck Level	Agency to Advise	Comment
Mid Brisbane >50 m3/s	Twin Bridges	20.0	SRC	Seqwater will advise of likely impact
Mid Brisbane >130 m3/s	Savages Crossing		SRC	Seqwater will advise of likely impact
Mid Brisbane >175 m3/s	Colleges Crossing		ICC	Seqwater will advise of likely impact
Mid Brisbane >430 m3/s	Burtons Bridge	19.6	SRC	Seqwater will advise of likely impact
Mid Brisbane >550 m3/s	Kholo Bridge	11.9	ICC	Seqwater will advise of likely impact
Mid Brisbane >1,000 m3/s	Minor Flood Level Savages Crossing		вом	Seqwater and BOM to discuss flows Seqwater will provide actual and

Management of the control of the con				projected
				Wivenhoe releases to BOM
Mid Brisbane >1,900 m3/s	Mt Crosby Weir Bridge	12.4	ICC	Seqwater will advise of likely impact
Mid Brisbane >2,000 m3/s	Fernvale bridge	33.8	SRC	Seqwater will advise of likely impact
Lower	Threshold of		вом, всс	Seqwater, BOM and
Brisbane	urban damage in		FIC	BCC FIC to discuss
>3,500 m3/s	ВСС			flows

- 5. At the first instance of the expectation of the trigger, contact by Seqwater will be by phone, thereafter advice will be via email.
- Sequater will provide daily email advice of the status to SRC, ICC, BOM and BCC when MId Brisbane flows are expected to reach or exceed 130 m3/s.
- 7. Seqwater will provide twice daily email advice of the status to SRC, ICC, BOM and BCC when Mid Brisbane flows are expected to reach or exceed 550 m3/s.
- 8. Nominated Councils officers are invited to contact the Flood Operations Centre for technical discussions at any time after mobilisation.
- 9. Segwater will advise when the FOC is demobilised.
- 10. In the event of email failure, contact will be by phone, if possible.

Items for further discussion

Will Segwater provide actual and projected Wivenhoe releases to Councils?

What are the triggers for phone contact to BCC FIC?

Regards

Terry Malone
Principal Hydrologist
Queensland Bulk Water Supply Authority trading as Sequeter



Level 3, 240 Margaret St, Brisbane City QLD 4000 Australia PO Box 16146, City East QLD 4002

Sent: Friday, 22 October 2010 3:01 PM To: Baddiley, Peter; Carroll, Don; Caswell, Evan; Charalambous, James; J.Perkins j.stuart Cc: Lavin, Chris; Scroope, Greg Subject: Friday Morning 29/10 - Technical Capability Meeting
As per our discussions today:
SEQWater/BOM/BCC have agreed to reconvene next Friday 29/10/2010
Time: 8:45 am to 12:30pm. Location: Green Square Level 2 (GSQ-L2A-4-10p)
Agenda as follows:
 Open discussion on communication between agencies, processing of information and data Suggestions or improvements to current systems/processes
Morning tea (10am) and lunch (12-12:30pm) will be provided.
The meeting will follow on from today's discussion (meeting notes attached).
This message has passed through an insecure network. Please direct all enquiries to the message author.
Your Anti-virus Service scanned this email. It is safe from known viruses. For more information regarding this service, please contact your service provider.
Information from ESET NOD32 Antivirus, version of virus signature database 5969
The message was checked by ESET NOD32 Antivirus.
http://www.eset.com

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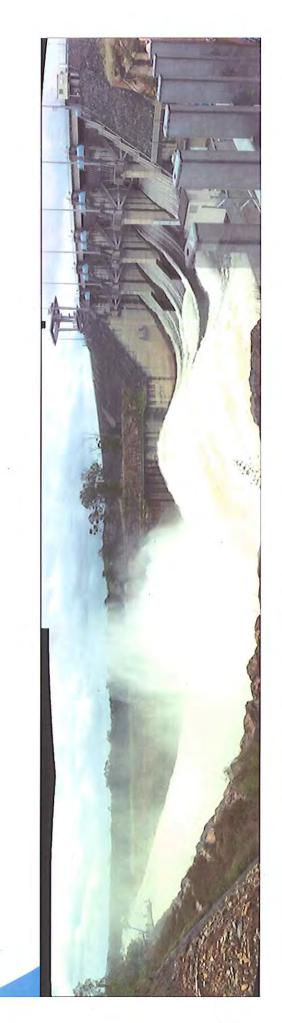


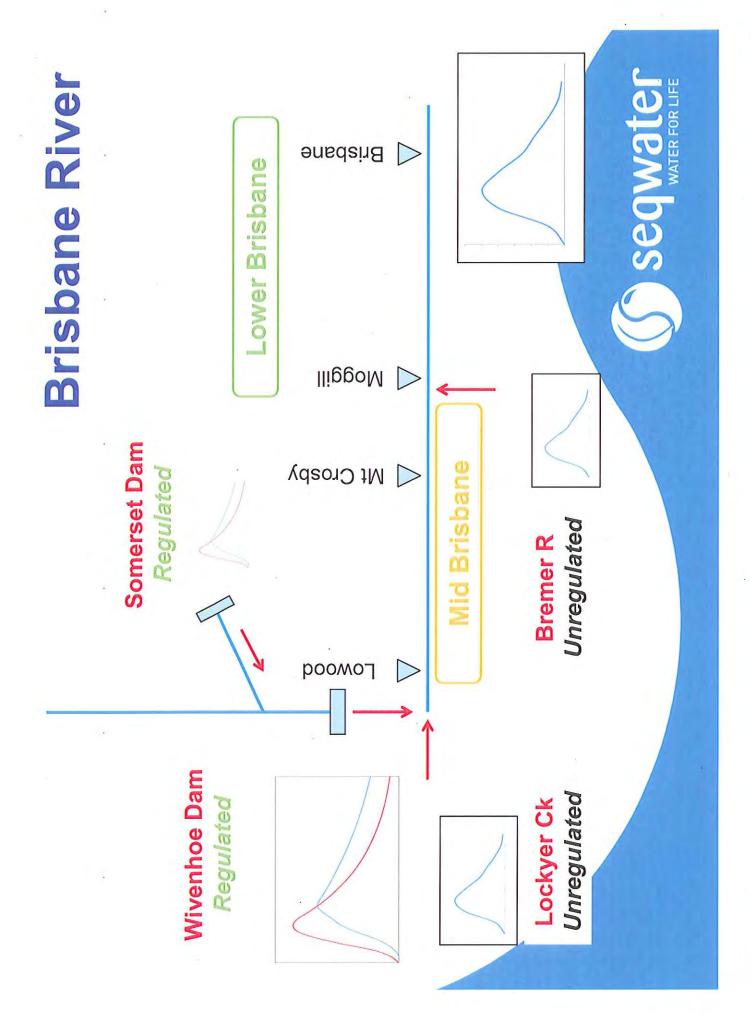
Roles, Responsibilities,
Technical Capability and
Future Directions in Flood
Management

BCC Meeting 22/10/2010

Responsibilities & Roles

- What we do
- Who we are
- Technical Capabilities
- Modelling floods
 - Future Directions
- FEWS
- Issues





Responsibilities

- All Seqwater Dams covered by Queensland Dam Safety Guidelines and Regulations
- Operation of North Pine, Somerset and Wivenhoe Dam covered by Water Supply Act 2008
- Manual of Operational Procedures
- Emergency Action Plans
- Other dams covered in-house operations



MANUAL

OPERATIONAL PROCEDURES

E.O.

FLOOD MITIGATION

AT

WIVENHOE DAM

2

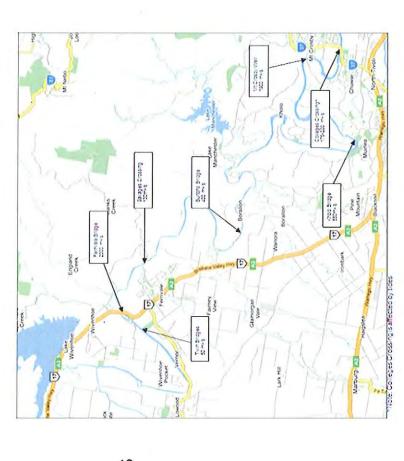
SOMERSET DAM

Revision 7 November 2009



Flood Manual Objectives

- Ensure the structural safety
- Optimum protection of urban areas
- Minimise disruption to rural life
- Retain the storage at FSL
- Minimise environmental impacts
- Drain in 7 days





Seqwater water

Triggers

APPENDIX B KEY REFERENCE GAUGES

		1974	Minor Flood	Tood	Moderate Flood	Hood	Major Flood	lood.
Location	Gauge Zero	Gauge Height	Cauge Beight	How	Cauge Height	Flow	Cauge Height	Flow
	mAHD	E	E	No.	E	E S	E	m ³ /s
Stanley River at Somerzet Dam	0.00	106.57	103.0		105.0		106.0	
Brishane River at Low ood	23.68	22.02	8.0		15.0		30.0	
Brisbane River at Lowood	22.74	1	98	ouo -	15.9	5	21.2	
Brishane River at Savages Crossing	18.43	13.79	00	8	16.0	ing's	21.0	9
Brishane River at Mt Crossby	0,00	26.74	0.11		13.0		21.0	
Berner River at Spswich	0,00	20.70	7.0		0.0		11.7	
Brishane River at Moggill	00.00	19.95	0.01		13.0		15.5	
Brishane River at Jindalee Bridge	0.00	14.10	0.9	4,000	078	5,000	100	6,500
Brishane River at City Gauge	0.00	5.45	17		3.6		33	,

Flows are approximate only and gauge beights are tide dependent in the lower reaches.

Roles

- Manager, Dam & Source Operations
- Rob Drury
- Principal Engineer, Dam Safety
- John Tibaldi
- Principal Hydrologist, Dam Safety
- Terry Malone
- Flood Operations Centre
- Contracted Headworks Operator Sunwater
- 2 Senior Flood Operations EngineersRob Ayre (SunWater), John Ruffini (DERM)
- 2 Flood Operations Engineers
- · Terry Malone, John Tibaldi (Seqwater)
- 12 Data Collectors (SunWater and Segwater)



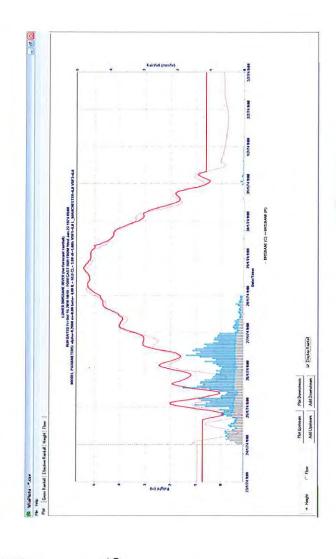
Flood Operations Centre Procedures

- FOC is mobilised whenever gate openings are expected
- FOC and dams are staff 24 hours until demobilisation
- Instructions for gate operations are issued and acknowledged by operators in writing
- Regular contact is maintained with BOM and affected Councils



Modelling Capabilities

- Hydrologic Modelling
- FloodCol & FloodOps
- Enviromon & URBS

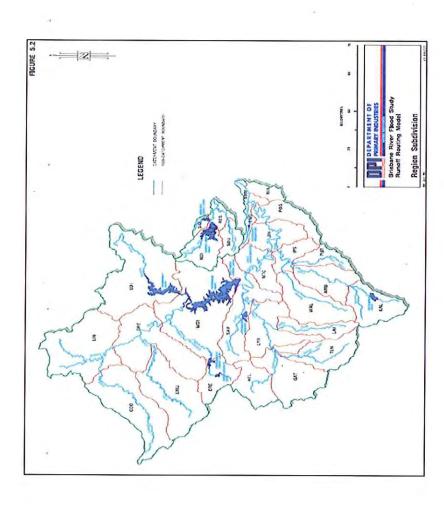




FloodCol & FloodOps

- FloodCol collects data from Seqwater owned stations
- FloodOps series of linked WT42 models
- Gate Operations –
 determined by spreadsheet model

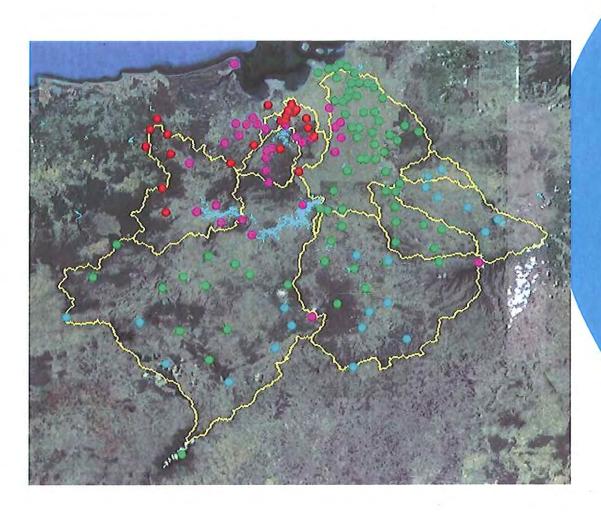
Seqwater ALERT Network 80 rainfall sensors 50 water level sensors





Enviromon & URBS

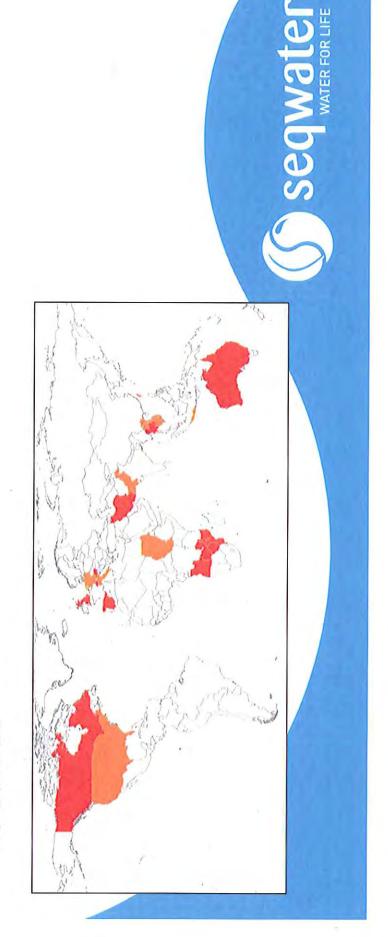
- Enviromon
- URBS series of linked models
- Baseflow
- Infiltration
- Dependent ratings
- Gate Operations determined by spreadsheet model





FEWS

- Forecasting Shell
- Integration of data from several sources
- Provides general functional utilities
- Open approach to integrating models and forecasting methods



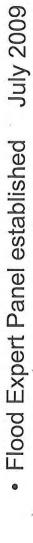
FEWS Modelling

- Wide range of models integrated
- Hydrological (snow models, rainfall-runoff, routing)
- Hydraulic (1D, 2D)
- Lookup models

				HEC-NAS
Model	Type	Supplier/Owner	Country	HEC-ResSim
SOBEK	Hydrodynamics, Water Quality, RR	Dellares	Netherlands	Snow17
SOBEK-2d	Linked 14/2d inundation modelling	Deltares	Netherlands	SAC-SMA
40 11				Unit-HG
DELF1-3D	ZU-3D Hydrodynamics	Deltares	Netherlands	LAG/K
Sacramento	Rainfall-Runoff	Deltares	Netherlands	SARROUTE
RIBASIM	Water distribution + Reservoir	Deltares	Netherlands	SSARRESV
REW	Distributed Rainfall-Runoff	Deltares	Netherlands	RESSNGL
DELFT3D	2/3D Hydrodynamics/ Water quality	Deltares	Netherlands	RES-J
Flux	1D Hydrodynamics	Scietec	Austria	PRMS
URBS	rainfall-runoff and hydrological routing	Don Caroll	Australia	SynHP

Model	Type	Supplier/Owner	Country
SISI	Hydrodynamic	HR/Halcrow	n K
PDW	Rainfall-Runoff	CEH	Ϋ́
TCM	Rainfall-Runoff	CEH	NX
KW	Routing (kinematic wave)	CEH	Ň
PACK	Snow Melt	CEH	χ'n
ARMA	Error Correction	CEH	ů. K
PRTF	Event Based RR	PlanB	'n
TRITON	Surge propagation/Overtopping	PlanB	ž
TWAM	2D Hydrodynamics	PlanB	ň
STF	Transfer functions	EA	UK
одод	Routing (layered Muskingum)	EA	č
MCRM	Rainfall-Runoff	EA	СK
Modflow96/VKD	3D groundwater	Deltares/Adam Taylor	Netherlands/UK
Mike11	Hydrodynamics	IHO	Denmark
NAM	Rainfall-Runoff	DHI	Denmark
TOPKAPI	Rainfall-Runoff	Univ, of Bologna	Italy
HBV	Rainfall-Runoff (inc snowmelt)	SHMI	Sweden
Vílo	Distributed Rainfall-Runoff	Vieux & Assiciates	USA
SWMM	Urban Rainfall-Runoff	nses	NSA
HEC-RAS	Hydrodynamic	USACE	USA
HEC-ResSim	Reservoir Simulation	USACE	USA
Snow17	Snow Melt	NWS	NSA
SAC-SMA	Rainfall-Runoff	NWS	NSA
Unit-HG	Unit-Hydrograph	NWS	NSA
LAGIK	Routing (hydrological)	NWS	USA
SARROUTE	Routing (hydrological)	NWS	USA
SSARRESV	Reservoir Simulation	NWS	USA
RESSNGL	Reservoir Simulation	NWS	USA
RES-J	(Multiple) Reservoir Simulation	NWS	NSA
PRMS	Rainfall-Runoff	Univ. of Karlsruhre	Germany
SynHP	Hydrodynamics	BfG	Germany

FEWS



BOM FEWS Pilot of Brisbane R July 2009

Expert Panel agrees to proceed Dec 2009

Contract signed with Deltares

May 2010

First version delivered

Initially WT42 and URBS

Nov 2010

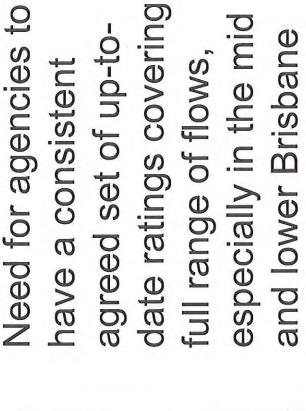
• Future ?

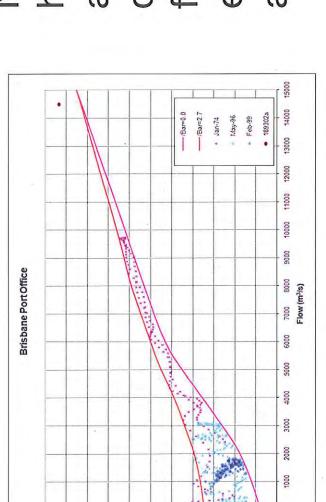
Continuous simulation

Hydrodynamic models



Ratings







Wivenhoe FSL Raising

- QWC instigated project to investigate impact of raising Wivenhoe FSL to 68
- 2005) to assess downstream impacts using existing Phase 1 – use existing design flood hydrology (WA
- Phase 2 modify rules to mitigate impacts
- Assess cost and benefits (hold point)
- Phase 3 re-derive design flows using new model and assess impacts
- Councils heavily involved in assessing & costing impacts

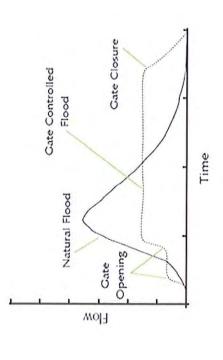


Triggers for Contact

Need for:

 an agreed set of triggers for initiating contact

- meaningful
- practical



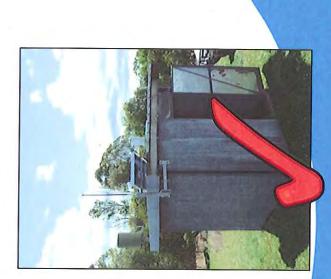
Difference between natural flows and releases from Wivenhoe



Gauge Ownership/Maintenance

Need to sort out:

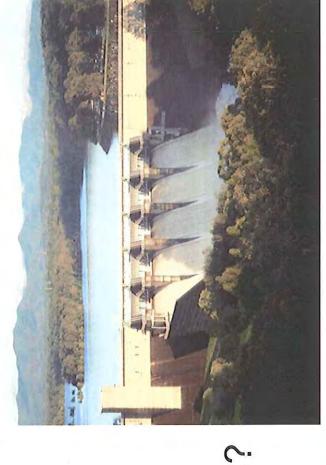
- Ownership
- Maintenance responsibilities
 - Installation standards











Questions?